

CONDOMINIUM DEVELOPMENT PLAN THE HAVEN AT LIBERTY GROVE

INDEPENDENCE, KENTON COUNTY, KENTUCKY

JULY, 2024

CONSULTING ENGINEER



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Lawrenceburg, IN 47025 Madison, IN 47250
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5 TOWER DRIVE
NEWPORT, KENTUCKY 41071
(513) 616-3913

ANR LOGISTICS LLC
289 MADISON PIKE
FT. MITCHELL, KENTUCKY 41017

ARLINGHAUS BUILDERS INC.
142 BARNWOOD DRIVE
EDGEWOOD, KENTUCKY 41017

PROJECT# ENG2311
CONDO DEVELOPMENT PLAN
INDEPENDENCE, KENTON COUNTY, KENTUCKY
APRIL, 2024
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ROBERT JOSEPH HREZO P.E.
KENTUCKY REGISTRATION #PE23254

DATE CERTIFIED

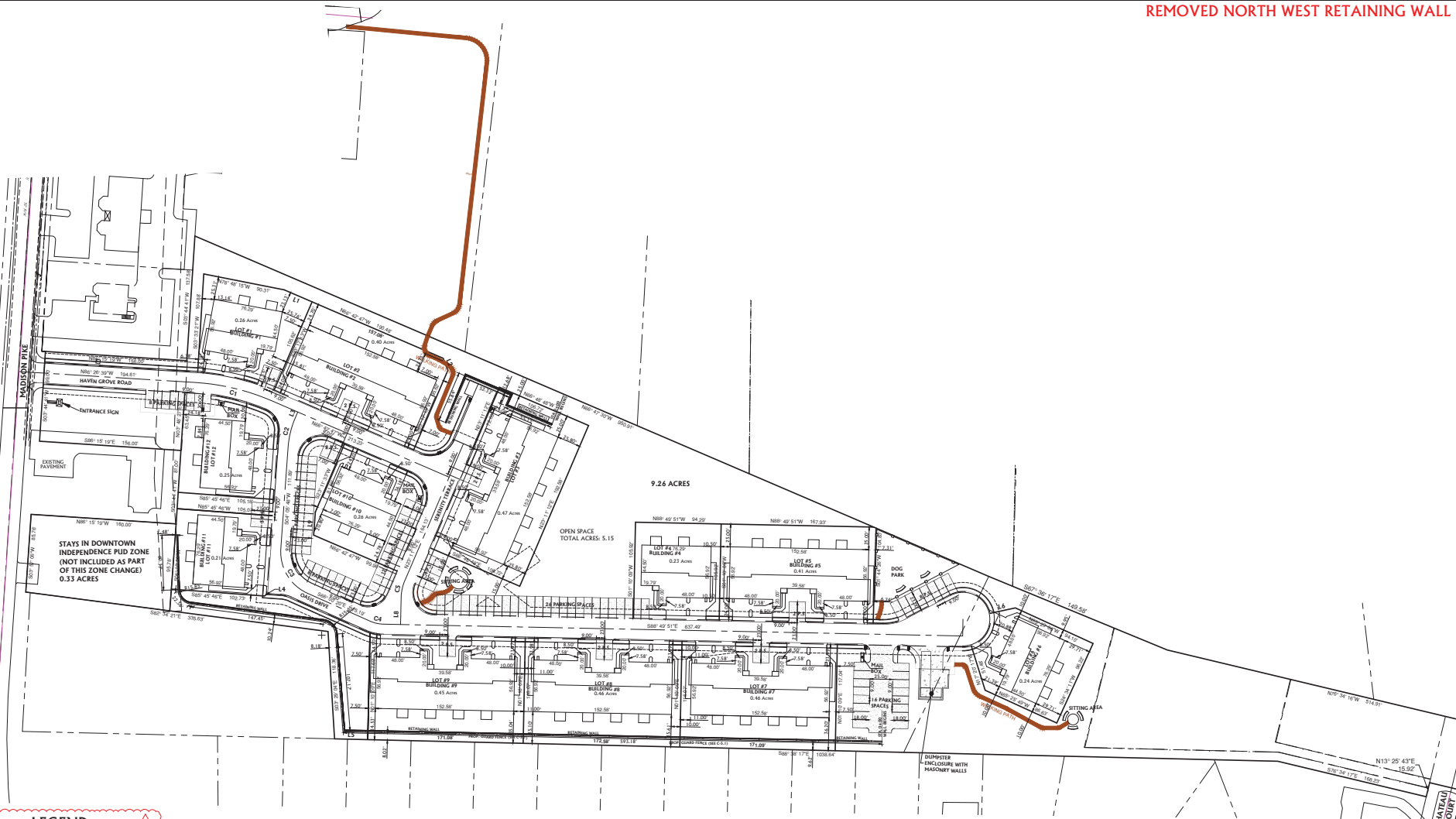
REMOVED NORTH WEST RETAINING WALL



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LEGEND

- [Symbol] LIGHT DUTY ASPHALT
- [Symbol] LIGHT DUTY CONCRETE
- [Symbol] HEAVY DUTY CONCRETE

PAVEMENT THICKNESS PER KENTON COUNTY STANDARDS. SEE TABLE 4-4-2 ON SHEET 5.3.

[Symbol] 4" ASPHALT WALKING TRAIL



OVERALL SITE LAYOUT PLAN

Curve Table

Curve #	Length	Radius	Delta	Chord Distance	Chord Length
C1	34.00	250.00	18.75	50° 34' 43.74"	85.47
C2	14.14	250.00	18.75	50° 34' 43.74"	33.24
C3	15.54	250.00	22.83	57° 21' 08.74"	37.42
C4	15.54	250.00	22.83	57° 21' 08.74"	37.42
C5	19.23	250.00	22.83	57° 21' 08.74"	37.42

Parcel Line Table

Line #	Length	Bearing
L1	21.00	S89° 47' 46.47"W
L2	14.14	S0° 00' 00.00"W
L3	2.00	S89° 17' 22.27"W
L4	1.51	S89° 00' 00.00"W
L5	36.42	S89° 38' 27.67"W
L6	18.00	S77° 50' 27.67"W
L7	18.00	S40° 00' 00.00"W
L8	23.52	N1° 10' 08.67"E
L9	20.43	N4° 00' 46.67"E

DATE: SEE RELEASE DATES

DRAWN BY: JCL

CHECKED BY: RJH

PROJECT: ENG 2311

THE HAVEN AT LIBERTY GROVE
 MADISON PIKE PARTNERS, LLC

ANRL LOGISTICS LLC: 289 MADISON PIKE, FT. MITCHELL, KENTUCKY 41017
 PALINGHAUS BUILDERS INC.: 142 BARNWOOD, KENTUCKY 41017

SITE LAYOUT PLAN
 C-1.1

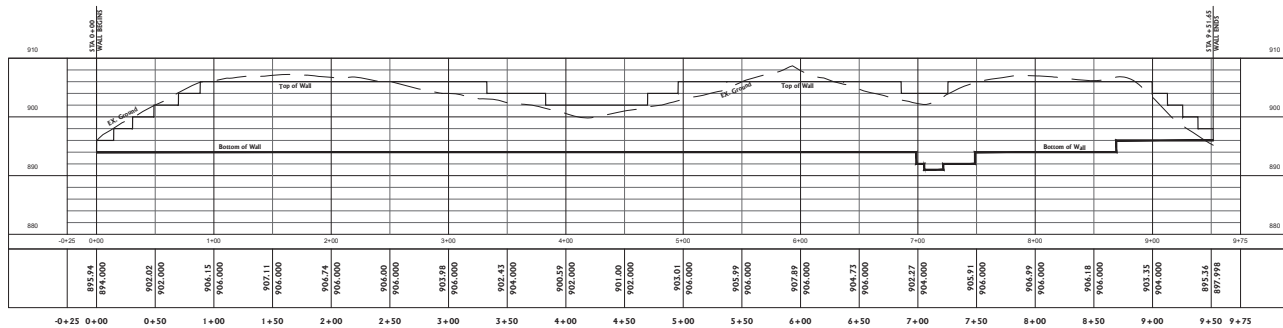
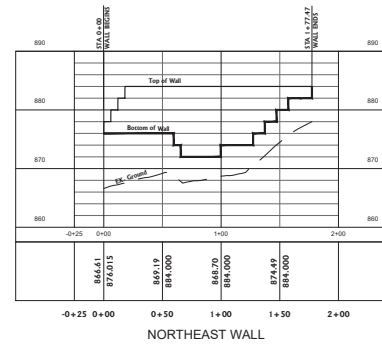
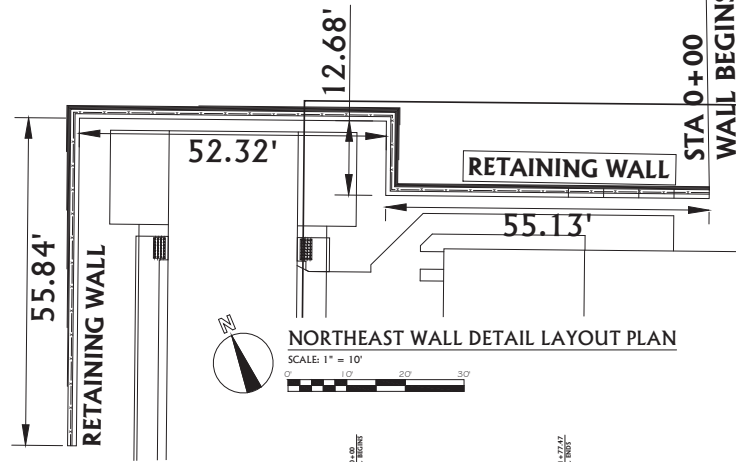


OFF SITE WALKING PATH DETAIL

SCALE: 1" = 30'



TRAIL CONNECTION WILL BE COMPLETED PRYOR TO ZONING PERMITS BEING ISSUED FOR THE NINTH BUILDING WITHIN THE DEVELOPMENT.



SOUTHERN WALL DETAIL LAYOUT PLAN

SCALE: 1" = 50'



WALL PROFILE VIEWS

VERTICLE SCALE: 1" = 10'
HORIZONTAL SCALE: 1" = 50'

REMOVED NORTH WEST RETAINING WALL

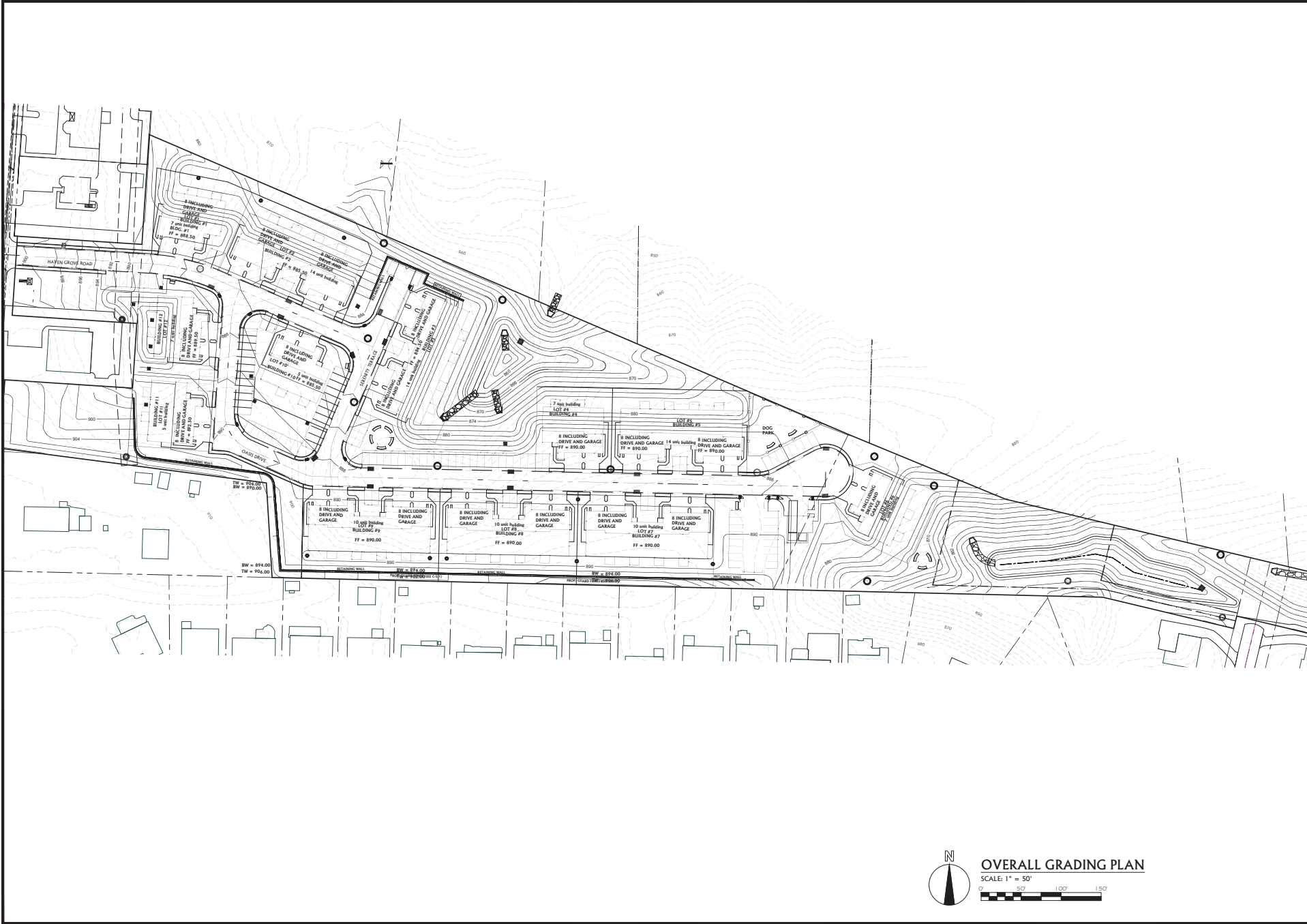


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NO.	REVISION	DATE
1	ISSUE FOR PERMIT	04/24/24
2		
3		
4		
5		
6		
7		
8		
9		
10		

DATE: SEE RELEASE DATES
DRAWN BY: JCL
CHECKED BY: RJH
PROJECT: ENG 2311

THE HAVEN AT LIBERTY GROVE
MADISON PIKE PARTNERS, LLC
ANRL LOGISTICS LLC 289 MADISON PIKE, FT. MITCHELL, KENTUCKY 41017
FALCONHUB BUILDERS INC. 142 BARNWOOD, KENTUCKY 41017
WALL DETAIL LAYOUT
C-1.2



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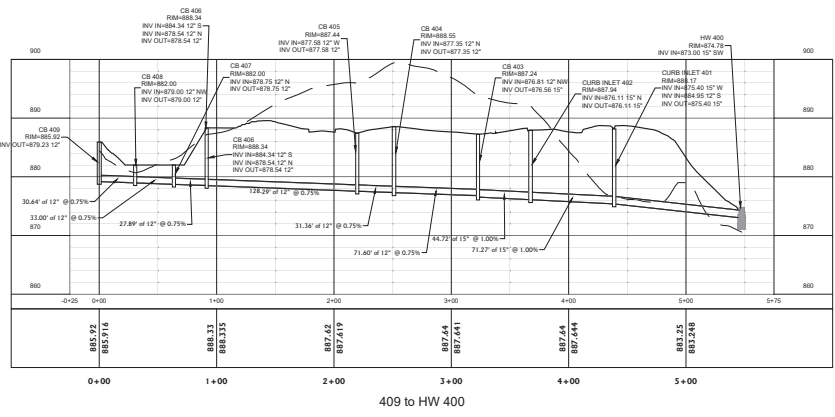
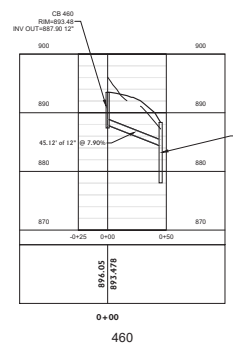
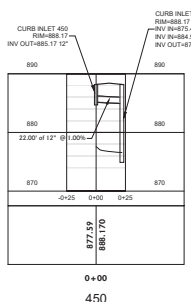
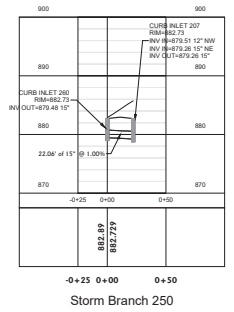
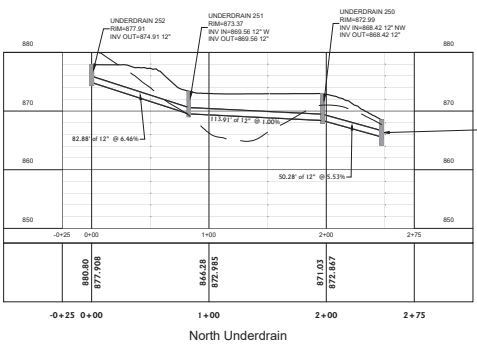
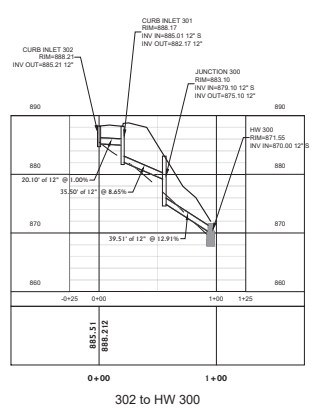
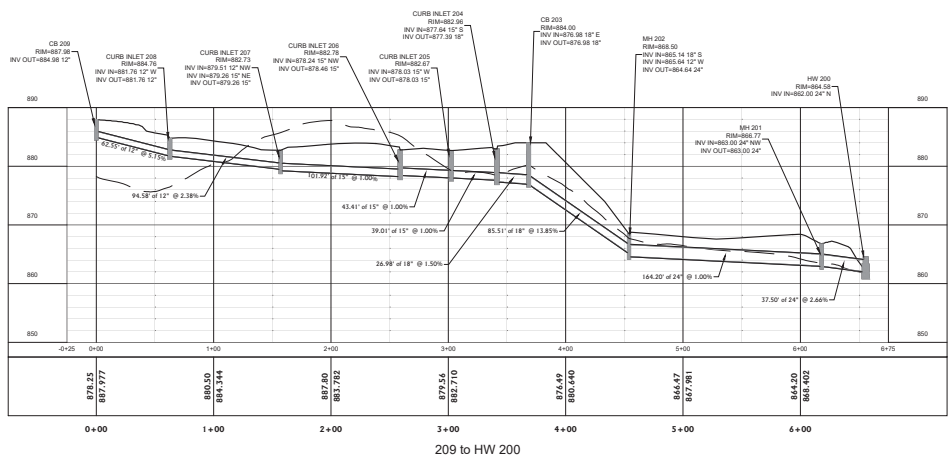
NO.	REVISION	DATE
1	ISSUE FOR PERMIT	10/14/2024
2		08/2024

THE HAVEN AT LIBERTY GROVE
MADISON PIKE PARTNERS, LLC

DATE: SEE RELEASE DATES
DRAWN BY: JCI
CHECKED BY: RJH
PROJECT: ENG 2311

ANR LOGISTICS LLC: 289 MADISON PIKE, FT. MITCHELL, KENTUCKY 41017
MADISON PIKE PARTNERS, LLC: 142 BARNWOOD, KENTUCKY 41017

GRADING PLAN
C-2.1



PROFILE VIEWS
VERTICAL SCALE: 1" = 10'
HORIZONTAL SCALE: 1" = 50'

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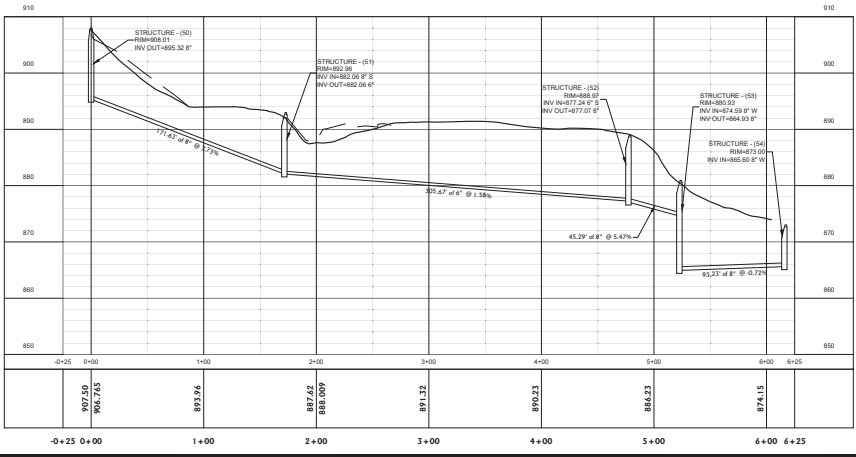
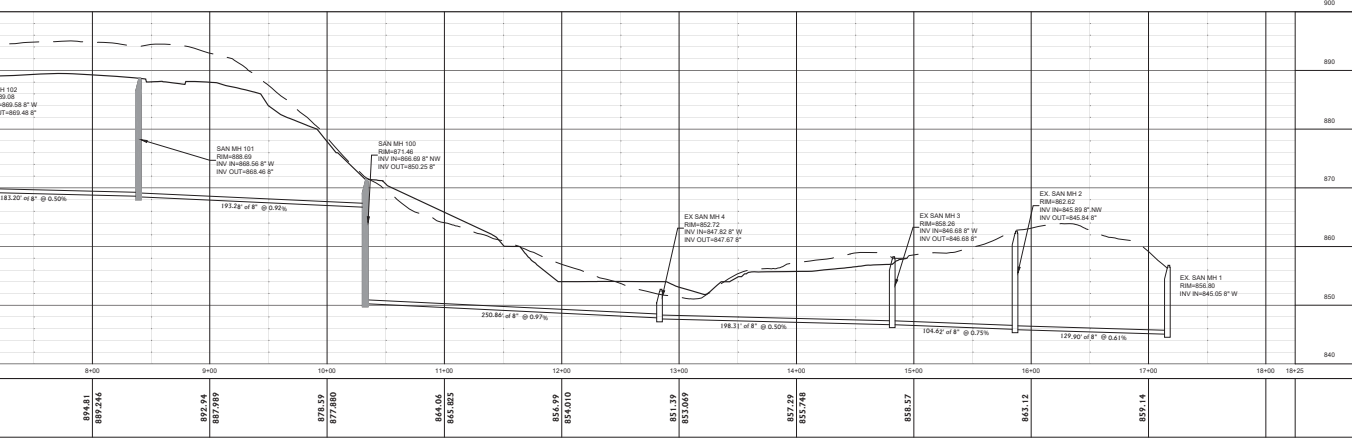
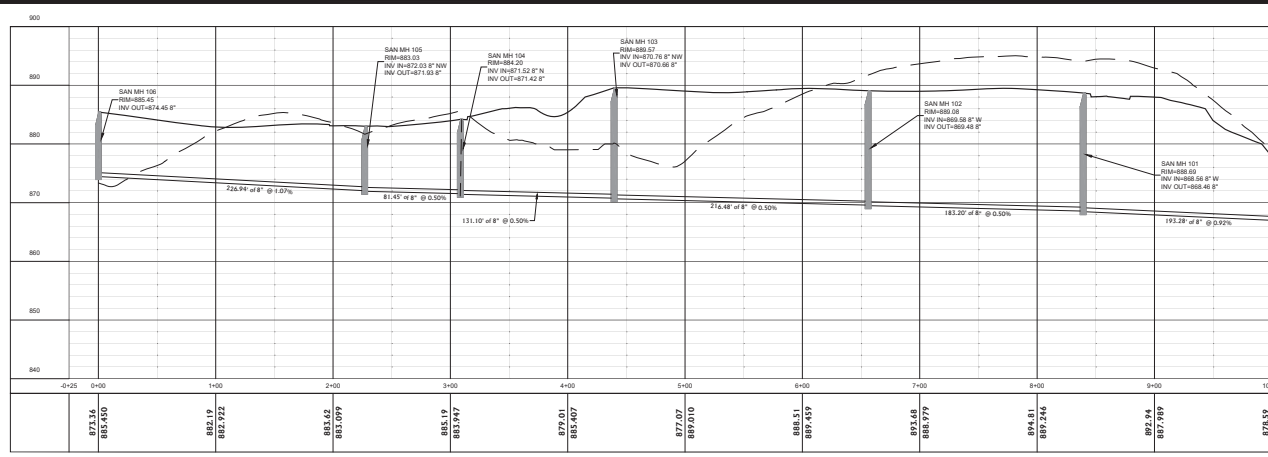
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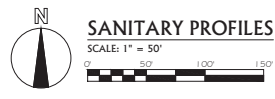
THE HAVEN AT LIBERTY GROVE
MADISON PIKE PARTNERS, LLC

ANRL LOGISTICS LLC: 289P MADISON PIKE, FT. MITCHELL, KENTUCKY 41017
KALINGHAUS BUILDERS INC.: 142 BARNWOOD, KENTUCKY 41017

DATE: SEE RELEASE DATES
DRAWN BY: JCL
CHECKED BY: RJH
PROJECT: ENG 2311



PROFILE VIEWS
 VERTICAL SCALE: 1" = 10'
 HORIZONTAL SCALE: 1" = 50'



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NO.	REVISION	DATE	BY
1	ISSUE FOR PERMITS	04-28	JKL
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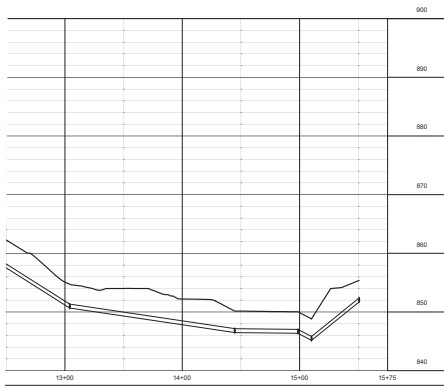
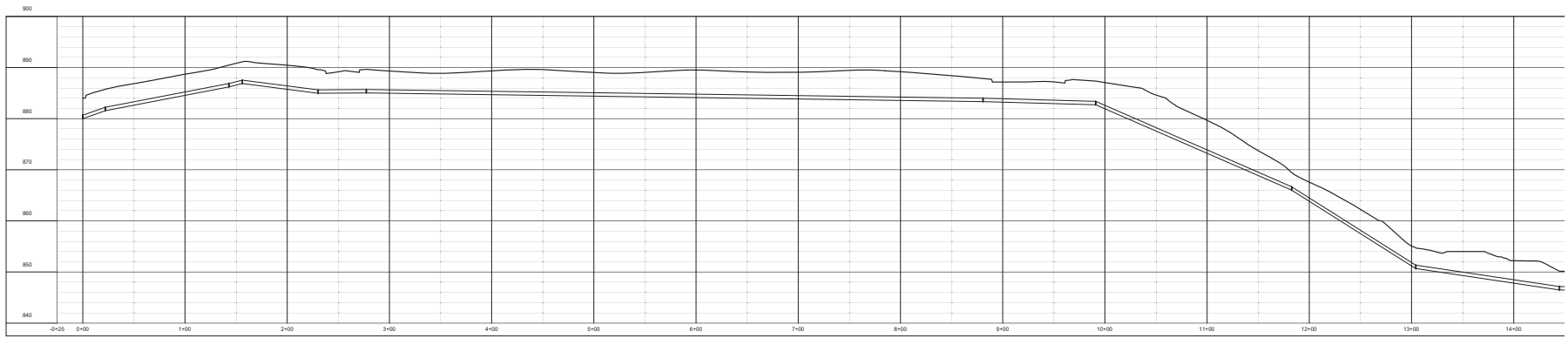
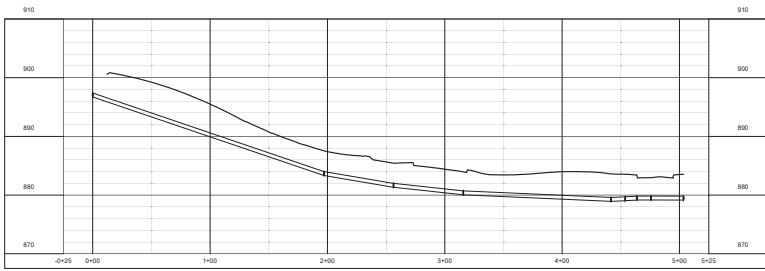
DATE: SEE RELEASE DATES
 DRAWN BY: JCL
 CHECKED BY: RJH
 PROJECT: ENG 2311

THE HAVEN AT LIBERTY GROVE
MADISON PIKE PARTNERS, LLC

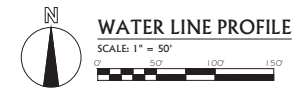
ANR LOGISTICS LLC 289 MADISON PIKE, FT. MITCHELL, KENTUCKY 41017
 FALINGHAUS BUILDERS INC. 142 BARNWOOD, KENTUCKY 41017

SANITARY SEWER PROFILES

C-3.4



PROFILE VIEWS
 VERTICAL SCALE: 1" = 10'
 HORIZONTAL SCALE: 1" = 50'



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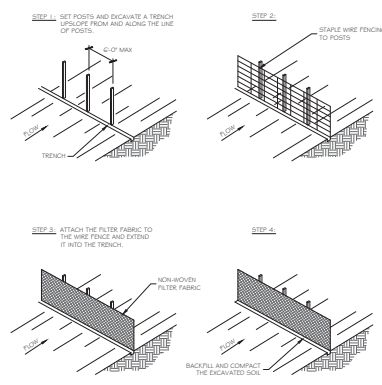
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REVISION	DATE
1	08/05/2014
2	08/28/14

PROJECT:	THE HAVEN AT LIBERTY GROVE
CLIENT:	MADISON PIKE PARTNERS, LLC
DESIGNER:	ANR LOGISTICS, LLC 289 MADISON PIKE, FT. MITCHELL, KENTUCKY 41017
CONTRACTOR:	FACHINGHAUS BUILDERS INC. 142 BARNWOOD, KENTUCKY 41017
DATE:	SEE RELEASE DATES
DRAWN BY:	JCL
CHECKED BY:	RJH
PROJECT:	ENG 2311

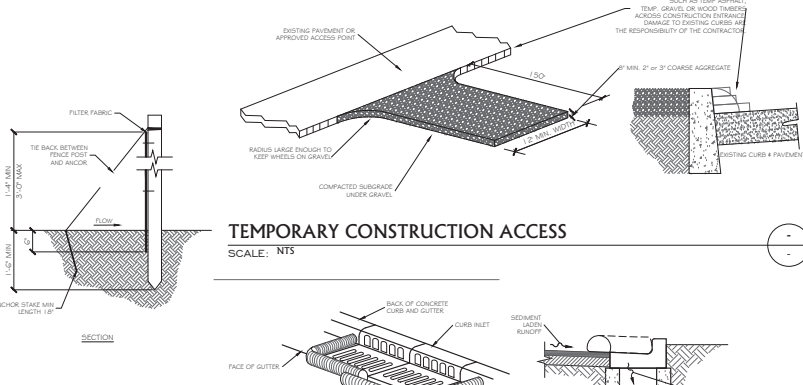
WATER
 LINE
 PROFILE

C-3.5



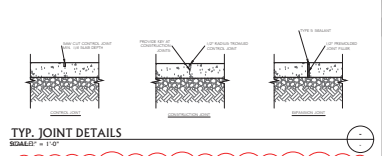
- NOTES:
- SILT FENCE SHALL BE PLACED AS CLOSE AS POSSIBLE TO EDGE OF CONSTRUCTION LIMITS ALONG CONTOUR LINES.
 - WOOD POSTS SHALL BE 1 1/2" X 1 1/2" SQUARE MINIMUM CUT, OR 1 3/4" DIAMETER MINIMUM, ROUND AND SHALL BE OF SOUND QUALITY HARDWOOD. STEEL POSTS MAY BE USED AND SHALL BE STANDARD T OR U SECTION WEIGHING NOT LESS THAN 1.0 POUNDS PER LINEAR FOOT.
 - FILTER FABRIC SHALL BE FASTENED SECURELY TO EACH FENCE POST WITH WIRE TIES OR STAPLES AT TOP AND MID-SECTION.
 - WHERE ENDS OF FILTER FABRIC COME TOGETHER, THEY SHALL BE OVERLAPPED, FOLDED AND STAPLED TO PREVENT SEDIMENT BYPASS.
 - SILT FENCE SHALL BE INSPECTED AFTER EACH RAINFALL EVENT AND MAINTAINED WHEN BULGES OCCUR OR WHEN SEDIMENT ACCUMULATION REACHES 50% OF THE FABRIC HEIGHT.

SILT FENCE BARRIER INSTALLATION
SCALE: NTS

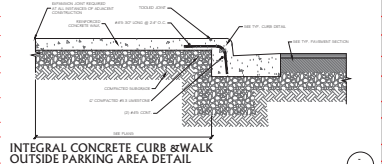


TEMPORARY CONSTRUCTION ACCESS
SCALE: NTS

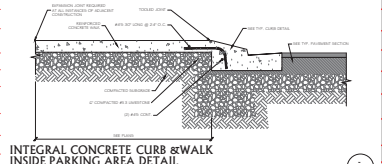
- PLACE AN ADEQUATE NUMBER OF SLOPEGUARD 3 ROLLS AROUND AN INLET TO PROVIDE COMPLETE PROTECTION. LEAVE APPROXIMATELY 2" GAPS BETWEEN THE SLOPEGUARD 3 ROLLS AND THE CURB. ENDS SHOULD OVERLAP ABOUT 12 INCHES.
- INSPECT INLET PROTECTION DEVICE BEFORE AND AFTER RAIN EVENTS, AND WEEKLY THROUGHOUT THE RAINY SEASON. DURING EXTENDED RAIN EVENTS, INSPECT AT LEAST ONCE EVERY 24 HOURS.
- REMOVE AND PROPERLY DISPOSE OF ACCUMULATED SILT AND DEBRIS TO ALLOW FOR PROPER FUNCTION OF DEVICE.



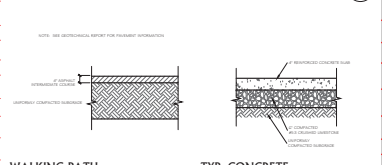
TYP. JOINT DETAILS
SCALE: 1" = 1'-0"



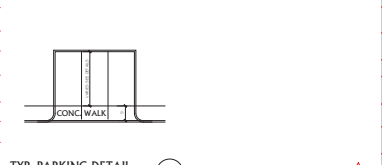
INTEGRAL CONCRETE CURB & WALK OUTSIDE PARKING AREA DETAIL
SCALE: 1" = 1'-0"



INTEGRAL CONCRETE CURB & WALK INSIDE PARKING AREA DETAIL
SCALE: 1" = 1'-0"



TYP. CONCRETE SIDEWALK
SCALE: 1" = 1'-0"



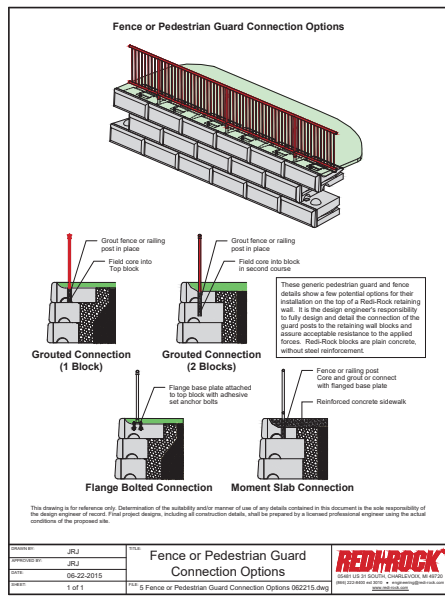
WALKING PATH ASPHALT PAVEMENT
SCALE: 1" = 1'-0"



TYP. PARKING DETAIL
SCALE: NTS



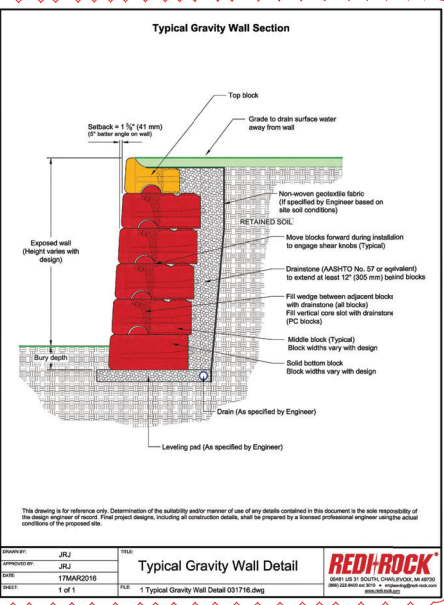
BARRICADE FENCE DETAIL
N.T.S.



Fence or Pedestrian Guard Connection Options
SCALE: N.T.S.

DATE: 06-22-2015
DRAWN BY: JRL
CHECKED BY: JRL
PROJECT: 5 Fence or Pedestrian Guard Connection Options 062215.dwg

RED-ROCK
CONCRETE PRODUCTS, INC.
1000 S. 1000 W. SUITE 100
MADISON, IN 47205
TEL: 317.273.2644
WWW.RED-ROCK.COM



Typical Gravity Wall Detail
SCALE: N.T.S.

DATE: 17-MAR-2016
DRAWN BY: JRL
CHECKED BY: JRL
PROJECT: 1 Typical Gravity Wall Detail 031716.dwg

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RETAINING WALL TEXTURE

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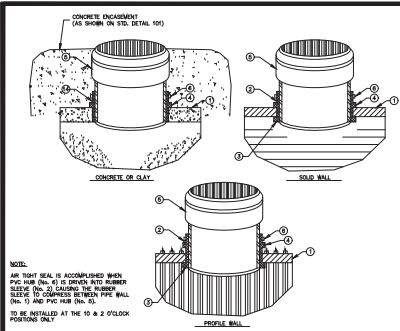
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DATE	BY	REVISION
11/20/2015	JRL	1

THE HAVEN AT LIBERTY GROVE
MADISON PIKE PARTNERS, LLC

ANR LOGISTICS, LLC: 289 MADISON PIKE, FT. MITCHELL, KENTUCKY 41017
KINGDALE BUILDINGS INC: 142 BANBROOK, KENTUCKY 41017

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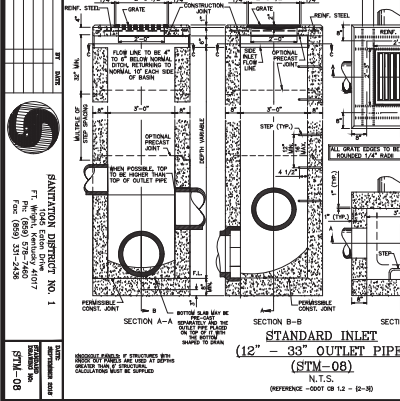


- NOTE: AIR TIGHT SEAL IS ACCOMPLISHED WHEN PIPE SEAL (No. 1) IS DRIVEN INTO RUBBER SLEEVE (No. 2) CLOSING THE RUBBER SLEEVE TO FORMER BETWEEN PIPE WALL (No. 1) AND PVC HUB (No. 3).
- NO. 1 SHALL BE INSTALLED AT THE 10 & 2 O'CLOCK PORTIONS ONLY.
1. MAINLINE PIPE WALL BROWN BRANCH LINE IS CONNECTED.
 2. COVER THE RUBBER SLEEVE COVERING BY JAWB-1448 DISPOSITION.
 3. COVER THE RUBBER SLEEVE COVERING BY JAWB-1448 DISPOSITION.
 4. RUBBER SLEEVE BRUSH IS INSERTED INTO THE RUBBER SLEEVE. THIS PREVENTS SWAP OUT ON THE INSIDE OF THE FITTING IN PLACE, NOT CREATING THE SEAL.
 5. RUBBER SLEEVE BRUSH IS INSERTED INTO THE RUBBER SLEEVE. THIS PREVENTS THE RUBBER SLEEVE FROM COMING OFF THE RUBBER SLEEVE. THE BRUSH IS TO BE REMOVED AFTER THE RUBBER SLEEVE IS IN PLACE.
 6. STAINLESS STEEL BANDS, PUT ON ABOVE FIGURE 4 IS AN GOOD PROTECTION.
- NOTE: PIPE PENETRATIONS SHALL BE CORE DRILLED.

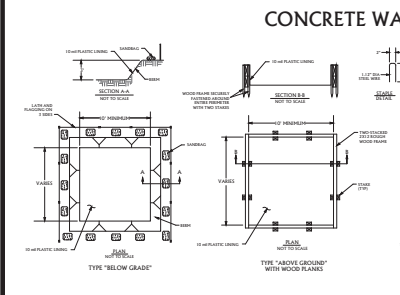
INSERTA TEE CONNECTION DETAILS
N.T.S.

REVISION	BY	DATE	DESCRIPTION
102		MARCH 2010	

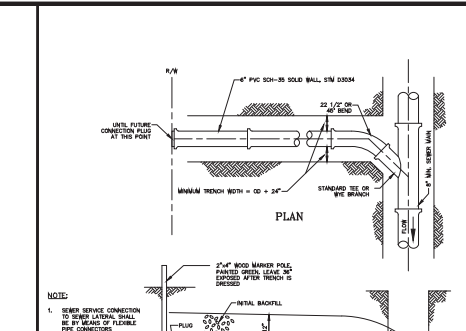
SANITATION DISTRICT NO. 1
1040 Edison Drive
FT. Wright, Kentucky 41017
Ph: (609) 578-7460
Fax: (609) 531-2436



STANDARD INLET (12" - 33" OUTLET PIPE) (STM-08)
N.T.S.



CONCRETE WASHOUT

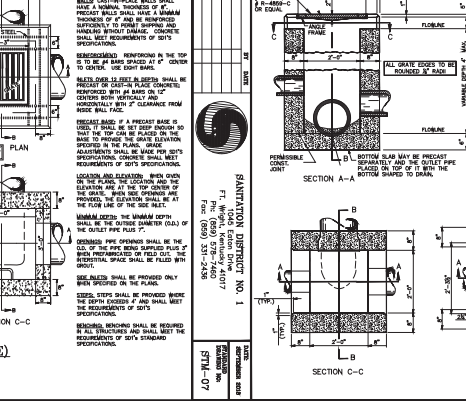


1. SEWER SERVICE CONNECTION TO MAINLINE SHALL BE BY MEANS OF FLEXIBLE PIPE CONNECTION (COUPLER OR JOINT TYPE) WITH GROUND PANELS OF ALL SEWER SERVICE LATERALS IF SHOWN ON DRAWING.
2. ALL LATERAL CONNECTIONS SHALL BE MADE BETWEEN THE 10 AND 11 O'CLOCK AND THE 1 AND 2 O'CLOCK POSITIONS TO THE SEWER MAIN.
3. LATERAL CONNECTION SHALL FOLLOW LOCAL PLUMBING CODES AND REGULATIONS FOR CONNECTIONS TO THE PUBLIC M/W.
4. LATERAL INSTALLATIONS SHALL FOLLOW ALL RELEVANT REQUIREMENTS IN DET. STANDARD OPERATIONAL SECTION 0202.

SEWER LATERAL INSTALLATION
N.T.S.

REVISION	BY	DATE	DESCRIPTION
120		SEPTEMBER 2008	

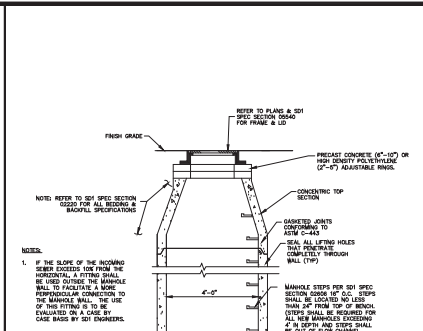
SANITATION DISTRICT NO. 1
1040 Edison Drive
FT. Wright, Kentucky 41017
Ph: (609) 578-7460
Fax: (609) 531-2436



STANDARD YARD DRAIN (12" - 21" OUTLET PIPE) (STM-07)
N.T.S.

- NOTES:
1. ACTUAL LAYOUT TO BE DETERMINED IN THE FIELD.
 2. A CONCRETE WASHOUT SKIN SHALL BE INSTALLED INSIDE OF THE TEMPORARY CONCRETE WASHOUT FACILITY.
 3. MATERIALS USED TO CONSTRUCT TEMPORARY CONCRETE WASHOUT FACILITY SHALL BE REMOVED FROM THE SITE OF THE WORK AND DISPOSED OF AS REQUIRED.
 4. HOLE, DEPENDENT ON OTHER GROUND DISTURBANCE, CAUSED BY THE EXCAVATION OF THE TEMPORARY CONCRETE WASHOUT FACILITY SHALL BE REPAIRED, RESEALED, AND STABILIZED TO PREVENT SINKING.
 5. HOLE IS TO BE LOCATED - 50 FT AWAY FROM WELLS/WATERWAYS UNLESS THERE IS AN OTHER PRACTICAL ALTERNATIVE.

CONCRETE WASHOUT SKIN DETAIL (OR EQUIVALENT)

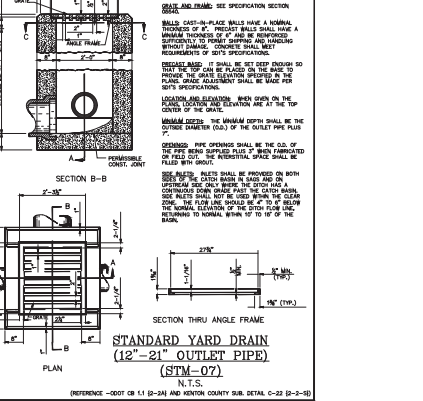


1. IF THE SLOPE OF THE INCOMING SEWER EXCEEDS THE SLOPE OF THE MANHOLE TO FACILITATE A MORE HORIZONTAL CONNECTION TO THE MANHOLE, THE SLOPE SHALL BE ESTABLISHED BY A CASE BY CASE BASIS BY THE ENGINEER.
2. PRECAST MANHOLES SHALL BE INSTALLED FOR SECTION 0202.
3. RINGING SHALL BE SHAVED TO MAINTAIN A SMOOTH SURFACE OF FLOOR FOR PIPE ANGLES. ALL RINGING SHALL BE REMOVED TO THE POINTS OF THE MANHOLE. NO RINGING SHALL BE ALLOWED TO REMAIN IN PLACE. ALL RINGING SHALL BE REMOVED TO THE POINTS OF THE MANHOLE. NO RINGING SHALL BE ALLOWED TO REMAIN IN PLACE.

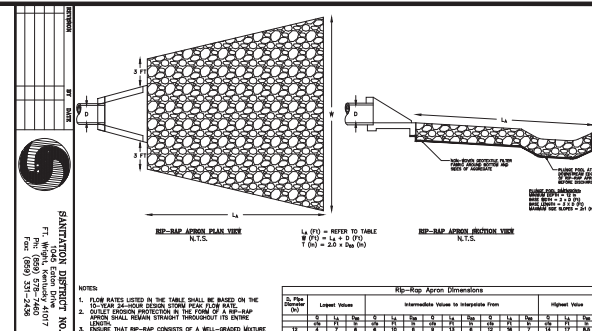
STANDARD MANHOLE
N.T.S.

REVISION	BY	DATE	DESCRIPTION
113		NOVEMBER 2008	

SANITATION DISTRICT NO. 1
1040 Edison Drive
FT. Wright, Kentucky 41017
Ph: (609) 578-7460
Fax: (609) 531-2436



CONCRETE WASHOUT



NOTE: 1. IF THE SLOPE OF THE INCOMING SEWER EXCEEDS THE SLOPE OF THE MANHOLE TO FACILITATE A MORE HORIZONTAL CONNECTION TO THE MANHOLE, THE SLOPE SHALL BE ESTABLISHED BY A CASE BY CASE BASIS BY THE ENGINEER.

NOTE: 2. PRECAST MANHOLES SHALL BE INSTALLED FOR SECTION 0202.

NOTE: 3. RINGING SHALL BE SHAVED TO MAINTAIN A SMOOTH SURFACE OF FLOOR FOR PIPE ANGLES. ALL RINGING SHALL BE REMOVED TO THE POINTS OF THE MANHOLE. NO RINGING SHALL BE ALLOWED TO REMAIN IN PLACE. ALL RINGING SHALL BE REMOVED TO THE POINTS OF THE MANHOLE. NO RINGING SHALL BE ALLOWED TO REMAIN IN PLACE.

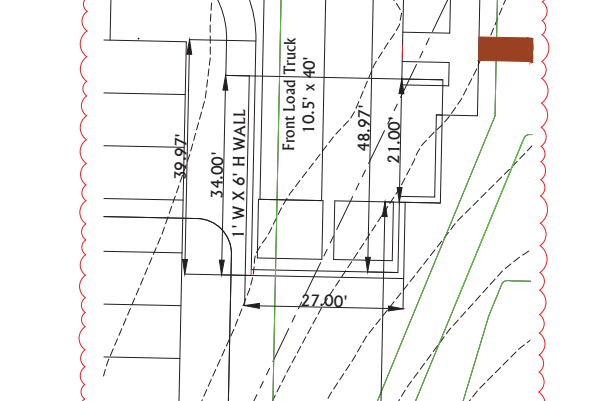
NOTE: 4. HOLE, DEPENDENT ON OTHER GROUND DISTURBANCE, CAUSED BY THE EXCAVATION OF THE TEMPORARY CONCRETE WASHOUT FACILITY SHALL BE REPAIRED, RESEALED, AND STABILIZED TO PREVENT SINKING.

NOTE: 5. HOLE IS TO BE LOCATED - 50 FT AWAY FROM WELLS/WATERWAYS UNLESS THERE IS AN OTHER PRACTICAL ALTERNATIVE.

STANDARD MANHOLE
N.T.S.

REVISION	BY	DATE	DESCRIPTION
113		NOVEMBER 2008	

SANITATION DISTRICT NO. 1
1040 Edison Drive
FT. Wright, Kentucky 41017
Ph: (609) 578-7460
Fax: (609) 531-2436



DUMPSTER ENCLOSURE
SCALE: 1" = 1'

- DATE: SEE RELEASE DATES
- DRAWN BY: JCA
- CHECKED BY: RHH
- PROJECT: ENG 2311

DETAIL SHEET

HEZRO
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REVISION

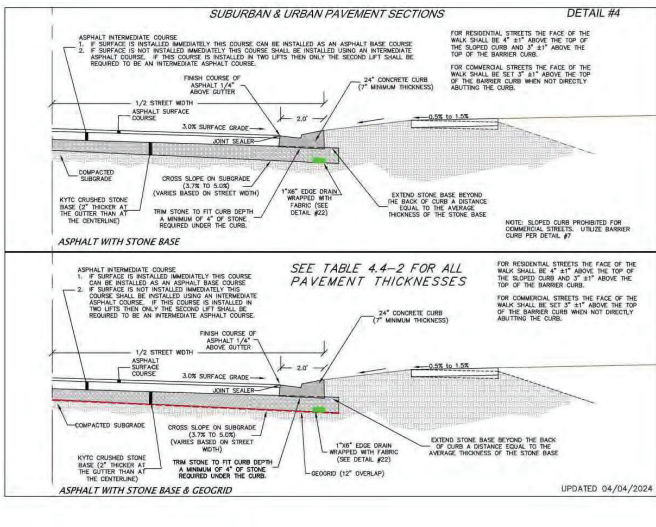
NO.	DATE	DESCRIPTION
1	12-21-08	ISSUE FOR CONSTRUCTION

REVISION

NO.	DATE	DESCRIPTION
1	12-21-08	ISSUE FOR CONSTRUCTION

THE HAVEN AT LIBERTY GROVE
MADISON PIKE PARTNERS, LLC
ANR LOGISTICS LLC, 289 MADISON PIKE, FT. MITCHELL, KENTUCKY 41017
ARLINGHAUS BUILDERS INC., 142 BARNWOOD, KENTUCKY 41017

C-5.2



SEE TABLE 4.4-2 FOR ALL PAVEMENT THICKNESSES

Table 4.4-2: Required Subdivision Street Thicknesses

Street Classification With Number of Lots Served ¹	Concrete	Concrete Over 4" Crushed Stone Base			Asphalt Over Crushed Stone Base (CSB)			Asphalt Over Crushed Stone Base (CSB) + Geogrid ⁴		
		Surface	Base ¹	Subgrade	Surface	Base	CSB ¹	Surface	Base	CSB ¹
Residential Local ≤199 Lots	7" ²	NA	1.25"	8"	1.5"	5"	7" ⁴	1.5"	3"	7" ⁴
Residential Subcollector 200 - 500 Lots	8" ²	NA	1.25"	8.75"	1.5"	6"	7" ⁴	1.5"	4"	7" ⁴
Residential Collector 501 - 1,000 Lots	9" ²	NA	1.25"	10.5"	1.5"	7.75"	7" ⁴	1.5"	5.75"	7" ⁴
Light Commercial ≤1 Million ESALS	Not Permitted	8.5" ³	Not Permitted	1.5"	8"	7" ⁴	1.5"	6"	7" ⁴	
Heavy Commercial/Industrial ≥1.5 Million ESALS	Not Permitted	10" ³	Not Permitted	1.5"	9.5"	7" ⁴	1.5"	7.5"	7" ⁴	

Per KYTC Specifications

¹ Pavement thickness. Varies from 1 inch less at centerline to 1 inch greater at gutter apron.

² Plain Concrete, tooled skewed transverse contraction joints without dowels (see Figure A.1-N, Detail #12 and Figure A.1-O, Detail #13).

³ Plain Concrete, with dowelled and sawed (iron-hawed) transverse contraction joints (see Figure A.1-M, Detail #11).

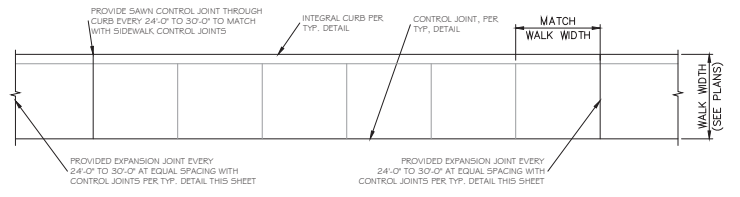
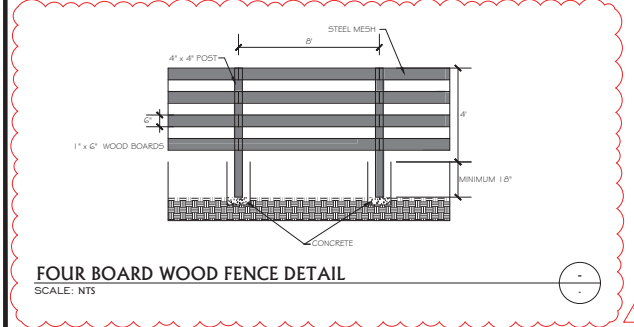
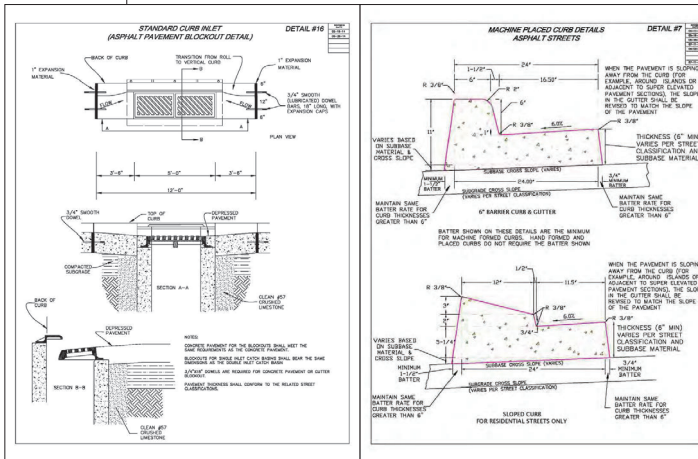
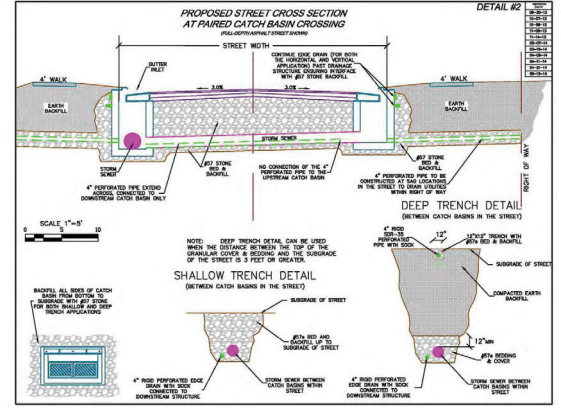
⁴ When the required thickness of aggregate base is no more than 8 inches for Crushed Stone Base (CSB) and the geogrid⁴ engineer⁵ has assumed that adequate compaction can be achieved, place the material in one lift.

⁵ Number of lots served for a particular Street is defined as the number of lots which that Street serves as the sole access⁶ where a number of Lots are served by more than one access. It is an approximation of the number of lots served that is equivalent to one access.

⁶ Example 1: A particular section Street serves as the sole access to less than 200 Lots. That Street would then be a Local Street.

⁶ Example 2: An area of existing and future residential development of 450 Lots is served by more than one access Street. Only those Streets that will carry traffic and ESAL loading higher than a Subdivision Street that provides sole access to more than 200 Lots will be classified as a subcollector Street.

⁷ Geogrid shall be Tensar TX5 triaxial geogrid.



- NOTES:
- CONSTRUCTION JOINTS TO BE LOCATED @ CONTROL JOINTS ONLY. PROVIDE CONTINUOUS KEY.
 - SPACING OF CONTROL JOINTS SHALL NOT EXCEED 6'-0". SPACINGS SHALL EQUALLY SPACED THROUGHOUT RUN.

HERZO Engineering, Inc.

Engineering Construction
Engineering Consulting
Full Architectural Services
Construction Management
Surveying Services

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THE HAVEN AT LIBERTY GROVE

MADISON PIKE PARTNERS, LLC

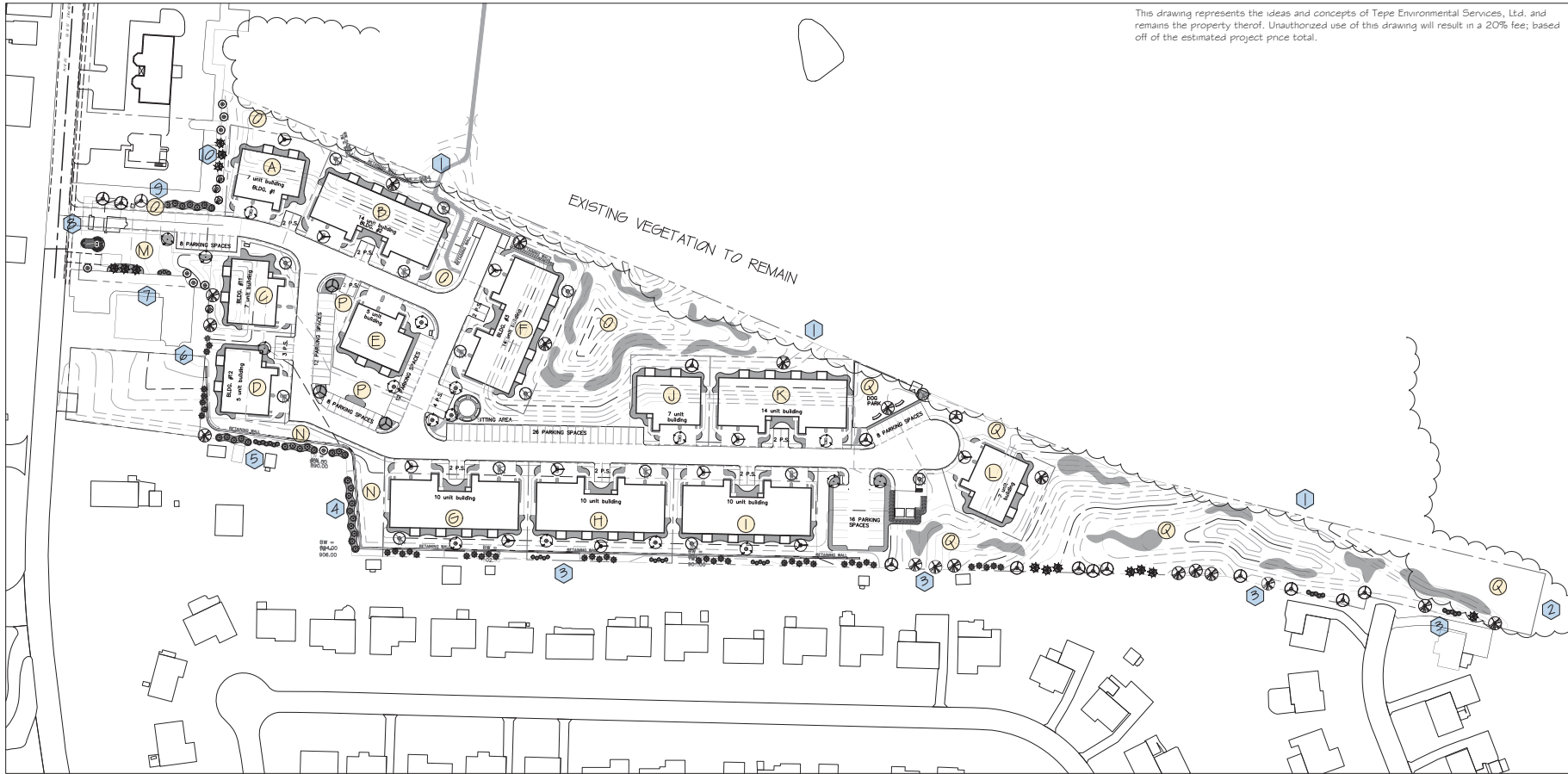
ANR LOGISTICS LLC, 289 MADISON PIKE, FT. MITCHELL, KENTUCKY 41017
MIDLANDHILLS BUILDERS INC. 174 BARNWOOD, KENTUCKY 41017

DATE: SEE RELEASE DAI
DRAWN BY: R
CHECKED BY: R
PROJECT: ENG 21

DETAIL SHEET

C-5.3

This drawing represents the ideas and concepts of Tepe Environmental Services, Ltd. and remains the property thereof. Unauthorized use of this drawing will result in a 20% fee; based off of the estimated project price total.



PERIMETER LANDSCAPE CALCULATIONS			
PROPERTY LINE	LENGTH	PLANTS REQUIRED	PLANTS PROVIDED
1	1479 in. ft.	3930 Trees 140 shrubs	Existing woodlands to remain
2	74 in. ft.	140 trees 536 shrubs	Existing woodlands to remain
3	1402 in. ft.	3504 trees 5740 shrubs	25 lg./ev. trees 40 lg shrubs
4	110 in. ft.	276 trees 478 shrubs	4 lg. trees 10 lg shrubs
5	116 in. ft.	290 trees 768 shrubs	2 lg./ev. trees 50 lg shrubs
6	177 in. ft.	934 trees 128 shrubs	4 lg./ev. trees 8 lg shrubs
7	156 in. ft.	392 trees 624 shrubs	3 lg trees 7 lg shrubs
8	99 in. ft.	No trees/shrubs required	No trees/shrubs provided
9	156 in. ft.	312 trees 624 shrubs	3 evergreen trees 7 lg shrubs
10	150 in. ft.	378 trees 652 shrubs	3 evergreen trees 8 lg shrubs

INTERIOR LANDSCAPE CALCULATIONS					
LOT	SIZE	TREES REQUIRED	TREES PROVIDED	SQFT REQUIRED	SQFT PROVIDED
A	1789 sq. ft.	1 lg. + 1 other	2 lg.	189 sq. ft.	189 sq. ft.
B	1700 sq. ft.	1 lg. + 2 others	3 lg.	190 sq. ft.	190 sq. ft.
C	1930 sq. ft.	1 lg. + 1 other	2 lg.	190 sq. ft.	190 sq. ft.
D	224 sq. ft.	1 lg. + 1 other	2 lg.	24 sq. ft.	107 sq. ft.
E	1140 sq. ft.	1 lg. + 1 other	2 lg.	134 sq. ft.	125 sq. ft.
F	2250 sq. ft.	1 lg. + 2 others	3 lg.	225 sq. ft.	243 sq. ft.
G	1930 sq. ft.	1 lg. + 2 others	3 lg.	190 sq. ft.	190 sq. ft.
H	2250 sq. ft.	1 lg. + 2 others	3 lg.	225 sq. ft.	225 sq. ft.
I	1930 sq. ft.	1 lg. + 2 others	3 lg.	190 sq. ft.	225 sq. ft.
J	2200 sq. ft.	1 lg. + 1 other	2 lg.	220 sq. ft.	155 sq. ft.
K	1830 sq. ft.	1 lg. + 2 others	3 lg.	183 sq. ft.	183 sq. ft.
L	1230 sq. ft.	1 lg. + 1 other	2 lg.	124 sq. ft.	123 sq. ft.
M	1930 sq. ft.	1 lg. + 1 other	2 lg.	225 sq. ft.	225 sq. ft.
N	1270 sq. ft.	1 lg. + 1 other	2 lg.	127 sq. ft.	130 sq. ft.
O	1820 sq. ft.	2 lg. + 4 others	3 lg. + 4 sm.	220 sq. ft.	420 sq. ft.
P	1500 sq. ft.	1 lg. + 1 other	2 lg.	225 sq. ft.	50 sq. ft.
Q	2127 sq. ft.	3 lg. + 4 other	3 lg.	222 sq. ft.	1247 sq. ft.

- PREPARATION AND PLANTING NOTES**
- LOCATE ALL UNDERGROUND UTILITIES PRIOR TO STARTING WORK THROUGH THE USE OF UTILITY PROTECT SERVICE.
 - PROTECT EXISTING UTILITIES, SIDEWALKS, PAVEMENTS, AND OTHER FACILITIES, AND LANDS AND EXISTING EXTERIOR PLANTS FROM DAMAGE CAUSED BY PLANTING OPERATIONS.
 - ALL PLANTING BEDS ARE TO BE PREPARED AS FOLLOWS:
 - REMOVE TOPSOIL TO A DEPTH OF 4" - 6" - REMOVE STONES
 - REMOVE TOPSOIL TO A DEPTH OF 4" - 6" AND FILL INTO THE UNDERLAYER WITH 18" GRANULAR AND STONE, SOUTH & NORTH
 - SPREAD COMPOST TO A DEPTH OF 4" - 6" AND FILL INTO THE UNDERLAYER WITH 18" GRANULAR
 - EDGE ALL LANDSCAPE BEDS & INDIVIDUAL TREE PITS 3" - 4" DEEP
 - IF THE CONTRACTORS OPTION METERS OR NOT TO STAKE A TREE, BUT IT IS ALSO THE CONTRACTORS RESPONSIBILITY TO ASSURE PLANTS REMAIN IN AN OPENLY POSITION UNTIL THE END OF THE WARRANTY PERIOD.
 - ALL PLANTING BEDS AND INDIVIDUAL TREES AND SHRUBS ARE TO BE MAINTAINED AT A DEPTH OF 18" AFTER PRE-EXISTENT WORKS BEFORE MAINTAIN PER MANUFACTURERS WRITTEN INSTRUCTIONS.
 - FOR ALL PLANT MATERIALS TAKE TO REMOVE AND OR WASTE FRANKIES ONLY, UNLESS OTHERWISE DIRECTED BY LANDSCAPE ARCHITECT.
 - IT IS THE CONTRACTORS RESPONSIBILITY TO:
 - KEEP ADJACENT PAVING AND CONSTRUCTION CLEAN AND MAINTAIN WORK AREA IN AN OPENLY CONDITION FOR REUSION OF PROJECT.
 - PROTECT EXTERIOR PLANTS FROM DAMAGE DUE TO LANDSCAPE OPERATIONS OPERATIONS BY OTHER CONTRACTORS AND TRADES, AND OTHERS. MAINTAIN PROTECTION DURING INSTALLATION.
 - REMOVE SURPLUS SOIL AND WASTE MATERIAL, INCLUDING EXCESS SOIL, UNDESIRABLE SOIL, TRUNKS AND BRANCHES, AND LEGALLY REMOVE OF THEM OFF OWNERS PROPERTY.



NOTE: In areas against a gravity retaining wall, trees cannot be installed without causing damage to the geogrid and wall systems. In this case, we propose the installation of the number of large shrubs equal to two times the number of trees required (in addition to all required shrubs). In keeping with the spirit and intent of the required plantings. This condition only exists on property line 4. For property line 5, sufficient space does not exist due to the retaining wall, so as many plants as possible have been provided.



Client:
Madison Pike Partners, LLC
5 Tower Drive
Newport, KY 41071

THE HAVEN AT LIBERTY GROVE
LANDSCAPE IMPROVEMENT PROJECT

Tepe Environmental Services, Ltd.
7021 Caves Warsaw Road
Newport, KY 41071
(513) 941-1400 - tepeenv.com

Issue Date:
29 JANUARY 2024

Revisions:
20 MARCH 2024

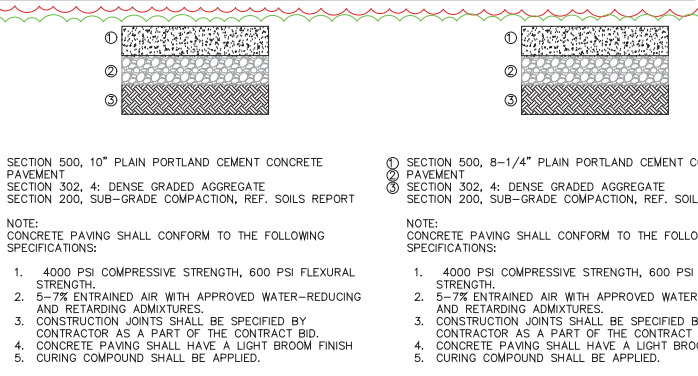
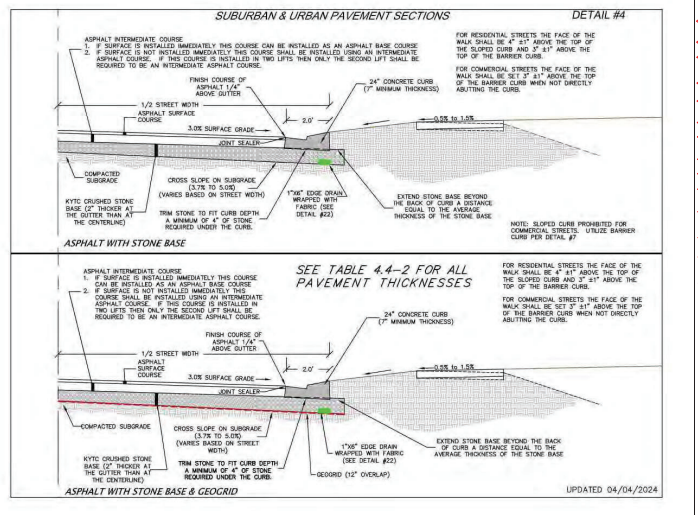
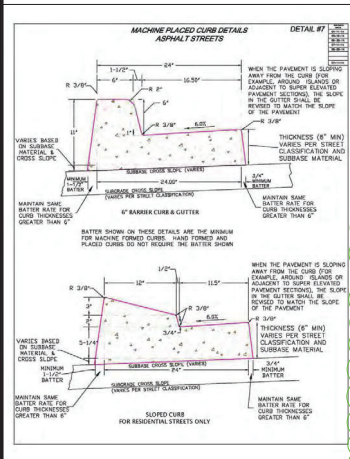
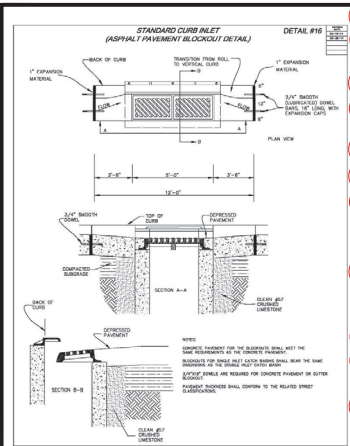
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LANDSCAPE PLAN

Designer:
KAYLA KNIGGA

Project Number:
24015

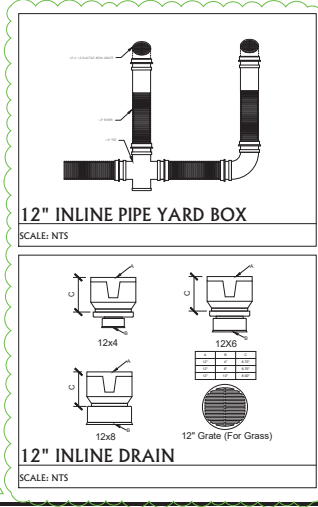
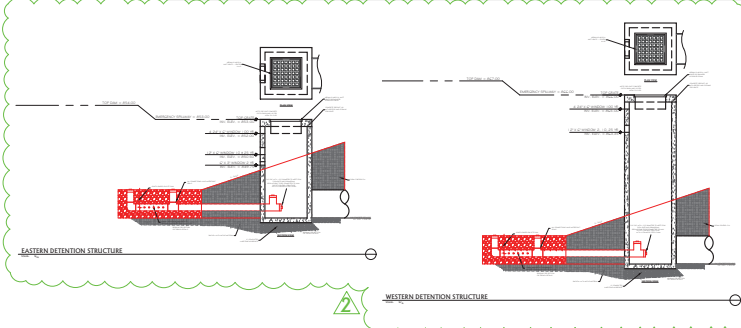
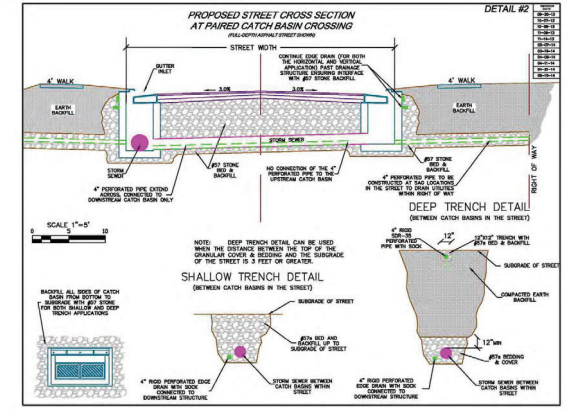
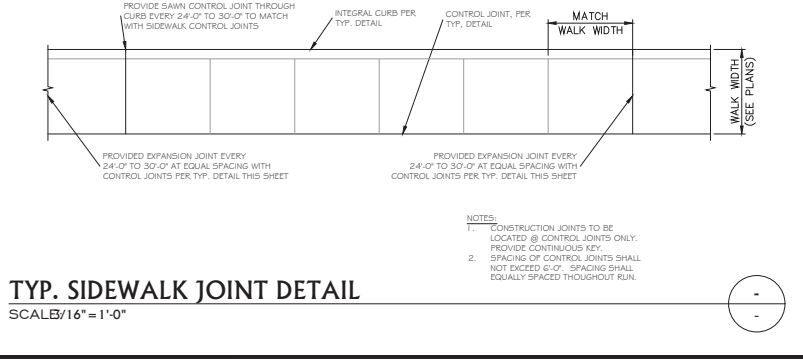
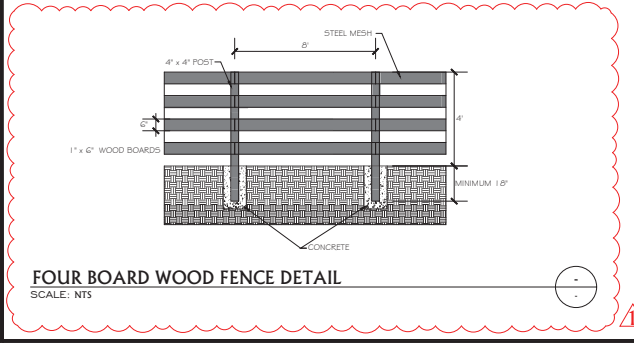
Scale:
1" = 60'-0"

Sheet:
L100



HEAVY CONCRETE PAVING SECTION NOT TO SCALE

LIGHT CONCRETE PAVING SECTION NOT TO SCALE



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REVISION	DATE
10-06-2024	10-14-24
10-06-2024	10-14-24

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THE HAVEN AT LIBERTY GROVE
MADISON PIKE PARTNERS, LLC
MADISON PIKE PARTNERS, LLC
1000 HANSON PIKE, FT. MITCHELL, KENTUCKY 41017
ARLINGHAUS BUILDERS INC. 142 BARNWOOD, KENTUCKY 41017
DATE: SEE RELEASE DATES
DRAWN BY: ICE
CHECKED BY: RJB
PROJECT: ENG 2311

DETAIL SHEET
C-5.3

July 29, 2024

Storm Water Calculations
Condominium Development at The Haven at Liberty Grove
Kenton County, Kentucky**Drainage Areas**Pre-Development

The pre-development watershed area for this site includes two water sheds, one that drains to the northwestern boundary and one that drains to the eastern boundary. The northwest watershed is approximately 7.43 acres, and the eastern watershed is approximately 2.26 acres. The watersheds converge at a creek offsite. Pre-Development conditions were analyzed and a weighted coefficient was calculated using the Sanitation District No. 1 Storm Water Rules and Regulations.

Post Development

The post development watershed area includes improvements of the development site, such as grading, buildings, and underground utilities. Open channel ditches as well as catch basins will be used to collect runoff from the improvements. There are two watersheds for the post development site draining to the similar locations as the pre development site. The post-development drainage area is increased for the eastern watershed (3.08 acres) and decreased for the north-western watershed (4.92 acres) increase. Post-Development conditions were analyzed and a weighted coefficient was calculated using the Sanitation District No. 1 Storm Water Rules and Regulations. The following charts shows calculated runoffs and provided detention to make Pre-Development conditions be the same or less than the Post Developments

RunoffPre-Development Runoff

The pre-development watershed area for the eastern watershed includes 2.26 acres of the development. The Rational Method is used to determine peak runoff going into the watershed. According to Section 506, Table 4 of the SD1 Rules and Regulations, a runoff coefficient is found. The land in this area is undisturbed giving a general runoff coefficient of 0.40 This is calculated through Hydrology Studio

Total Predeveloped Drainage Area: 2.26 Ac.

Runoff Coefficient: 0.40

Time of Concentration: 25 minutes

The pre-development watershed area for the northwestern watershed includes 7.34 acres of the development. The Rational Method is used to determine peak runoff going into the watershed. According to Section 506, Table 4 of the SD1 Rules and Regulations, a runoff coefficient is found. This is calculated through Hydrology Studio

Total Predeveloped Drainage Area: 7.34 Ac.

Runoff Coefficient: 0.40

Time of Concentration: 25 minutes

Using Section 503, Table 3 of the SD 1 Rules and Regulations, The Pre-Development conditions are as follows where:

$$I = B/(T_c+D)^E$$

Storm Frequency	B	D	E
3-month	35.0000	9.00	0.8950
2-Year	41.5018	7.62	0.8000
5-Year	54.0284	8.85	0.8110
10-Year	65.6903	9.80	0.8240
25-Year	43.9684	6.33	0.7020
50-Year	40.0295	5.23	0.6590
100-Year	34.3934	3.78	0.6050

Q-Critical:

According to Section 709 of the SD 1 Rules and Regulations, the 2-Year Frequency storm event shall be equal to or less than Q-Critical. This is approximated as 0.4 cfs per acre of disturbance within the pre-developed drainage area plus the 2-year peak from the remaining undisturbed pre-developed drainage area to each outlet/discharge leaving the project site.

Peak Runoff (East)

$$Q_2 = \text{Q-Critical} = .4(2.26) = 0.904 \text{ cfs}$$

$$Q_{10} = 3.586 \text{ cfs}$$

$$Q_{25} = 3.984 \text{ cfs}$$

$$Q_{50} = 4.306 \text{ cfs}$$

$$Q_{100} = 4.582 \text{ cfs}$$

Peak Runoff (Northwest)

$$Q_2 = \text{Q-Critical} = .4(7.34) = 2.94 \text{ cfs}$$

$$Q_{10} = 11.65 \text{ cfs}$$

$$Q_{25} = 12.94 \text{ cfs}$$

$$Q_{50} = 13.99 \text{ cfs}$$

$$Q_{100} = 14.88 \text{ cfs}$$

Post Development Runoff:

The post-development uncontained flow leaving the storm system is 1.59 acres of the watershed. This runoff is apart of our site basin but is not contained in our detention system. The Post Development conditions are as follows, using similar guidelines from the pre-development conditions as this area is graded but doesn't have impervious services.

$$\text{Runoff Coefficient} = 0.45$$

$$\text{Drainage area} = 1.59 \text{ acres}$$

$$\text{Time of Concentration} = 25 \text{ min. (Estimated Time of Concentration)}$$

$$Q_2 = 1.828 \text{ cfs}$$

$$Q_{10} = 2.523 \text{ cfs}$$

$$Q_{25} = 2.803 \text{ cfs}$$

$$Q_{50} = 3.030 \text{ cfs}$$

$$Q_{100} = 3.224 \text{ cfs}$$

The estimated post development watershed area to the Eastern Detention Pond includes 3.08 acres of improvement area. Open channel ditches, as well as catch basins will be used to collect runoff from the improvements. This will provide natural infiltration and help extend the time of concentration. This runoff will be directed to storm basins and outfall headwalls. The Post Development conditions are as follows, using the Modified Rational Method from the pre-development conditions.

Runoff Coefficient. = 0.81 (Multi-Family Residential)
 Drainage area = 3.08 acres
 Time of Concentration = 11 min. (Estimated Time of Concentration)

The estimated post development watershed area to the Eastern Detention Pond includes 3.08 acres of improvement area. Open channel ditches, as well as catch basins will be used to collect runoff from the improvements. This will provide natural infiltration and help extend the time of concentration. This runoff will be directed to storm basins and outfall headwalls. The Post Development conditions are as follows, using the Modified Rational Method from the pre-development conditions.

Runoff Coefficient. = 0.81 (Multi-Family Residential)
 Drainage area = 4.92 acres
 Time of Concentration = 13 min. (Estimated Time of Concentration)

The Peak Runoff rates to Detention pond are listed below. This was determined via Hydrology Studio, based on the numbers from the Pre-Development Run-off and Post Development conditions.

$Q_2 = 10.71$ cfs
 $Q_{10} = 14.75$ cfs
 $Q_{25} = 16.35$ cfs
 $Q_{50} = 14.71$ cfs
 $Q_{100} = 15.74$ cfs

Storage Requirements

The storage requirement was calculated using the NRCS Rainfalls and volumes were maximized to keep the runoff rate less than or equal to the pre-development rate. The detention pond design will provide more than the required storage. Detailed below is the stage storage for the proposed detention pond:

Eastern Pond

Stage (ft)	Elevation (ft)	Contour Area (sq. ft.)	Incr. Storage (cu. ft.)	Total Storage (cu. ft.)
0.00	848.00	1902	0.000	0.000
2.00	850.00	5362	7264	7264
4.00	852.00	9095	14456	21721
5.00	853.00	11,124	10109	31830
6.00	854.00	13,183	12153	43983

Western Pond

Stage (ft)	Elevation (ft)	Contour Area (sq. ft.)	Incr. Storage (cu. ft.)	Total Storage (cu. ft.)
0.00	858.00	221	0.000	0.000
2.00	860.00	948	1168	1168
4.00	862.00	2154	3101	4269
6.00	864.00	4053	6206	10476
8.00	866.00	6487	10540	21016
9.00	867.00	7945	7216	28232

The 100 year storm event will be able to be held in the pond as designed. A safety factor is built in to allow for the possibility of a clogged inlet. The outlet structure will have a grated opening in the top and the dam is an additional 1'-0" above the structure.

Detention Structure:

The Detention Structure was designed in order to keep the post-development release rates less than the pre-development release rates. In this case, the target release rate should be the pre-developed conditions subtracted from the uncontained post-developed conditions.

Best Management Practices:

For the best management practices, the estimated percentage of impervious surfaces post-development is equal to approximately 75%, and the site will be considered a new development of the area. The Calculations are as follows:

$$Rv = .009 * 75 + 0.05 = 0.725$$

$$VWq \text{ (NW pond)} = Rv * .8 \text{ in} * 4.92 \text{ ac} * 3630 = 10359 \text{ ft}^3$$

$$VWq \text{ (E pond)} = Rv * .8 \text{ in} * 4.92 \text{ ac} * 3630 = 6484 \text{ ft}^3$$

A Drawdown time for the VWq is approximated at 36 to 48 hours.

A 6-in perforated will be used to filter sediment and the storm water will be released into the NW detention structure using a 1.25" Orifice. This will release the W/QV at a rate of 0.14 cfs with a Drawdown time of 36 hrs.

A 6-in perforated will be used to filter sediment and the storm water will be released into the NW detention structure using a 1.25" Orifice. This will release the W/QV at a rate of 0.07 cfs with a Drawdown time of 43 hrs.

Final Design:

Using the Best Management Practices described, the final structure was designed. A proposed Outlet structure was designed using the Hydrology Studio Program and is illustrated in C-5.3. The following is a summary of the resulting total release rates after the post-release conditions are established

Q Total Release:

$$Q_2 = 3.41 \text{ cfs}$$

$$Q_{10} = 5.34 \text{ cfs}$$

$$Q_{25} = 6.98 \text{ cfs}$$

$$Q_{50} = 9.29 \text{ cfs}$$

$$Q_{100} = 10.11 \text{ cfs}$$

Discharge from Detention Basin

The discharge rate will be controlled by the size of the orifices in the outlet structure and is a multi-stage release design. This design will allow for low flows to pass through and for sediments to settle during larger events. See plan details for outlet structure design. The downstream outlet from this pond is the south corner of the flowing to an existing waterbody

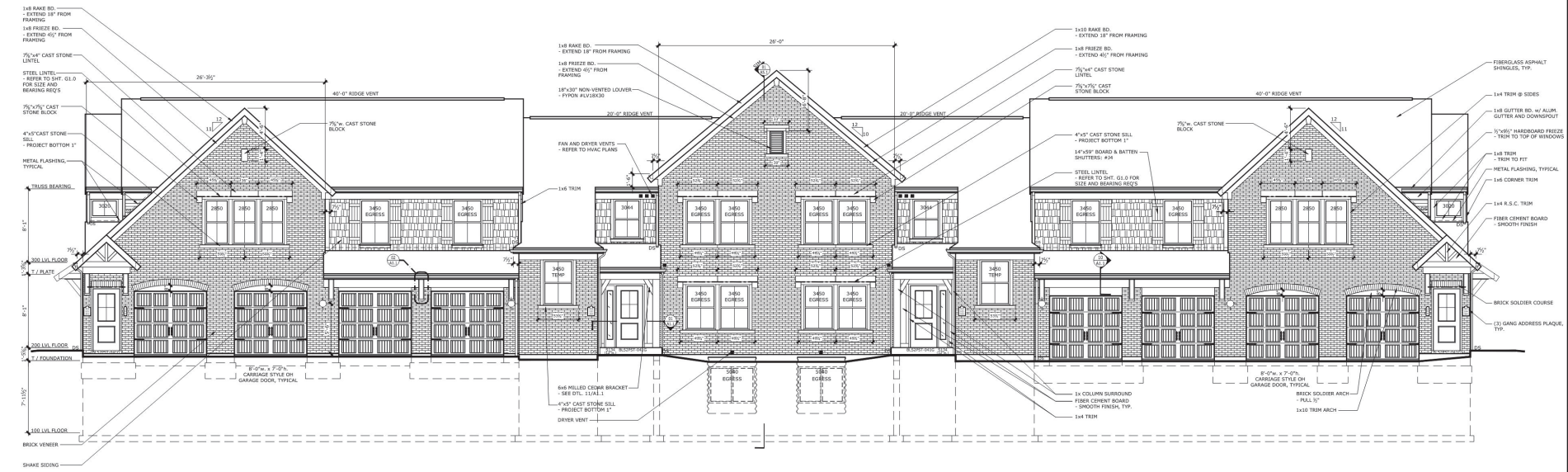
The proposed outlet pipe from the NW outlet structure in the pond will be a 47.51' of 24" Concrete pipe laid at 8.42% grade and will discharge less than the pre-development runoff per design grades. The proposed outlet pipe from the E outlet structure in the pond will be a 47.86' of 24" Concrete pipe laid at 12.06% grade and will discharge less than the pre-development runoff per design grades. The detention pond design will include a storage volume of more than the computed required storage and will be constructed of earth. Rip-Rap rock liner will be used from the outlet headwall to slow velocities and help filter sediments.

Pipe Hydraulics: See Additional Sheet

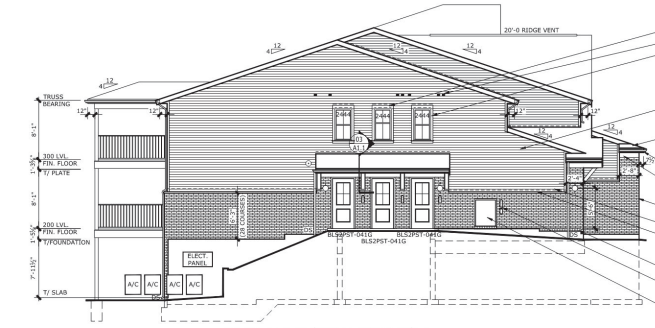
Sincerely,

Robert J. Hrezo, P.E.
Hrezo Engineering, Inc.

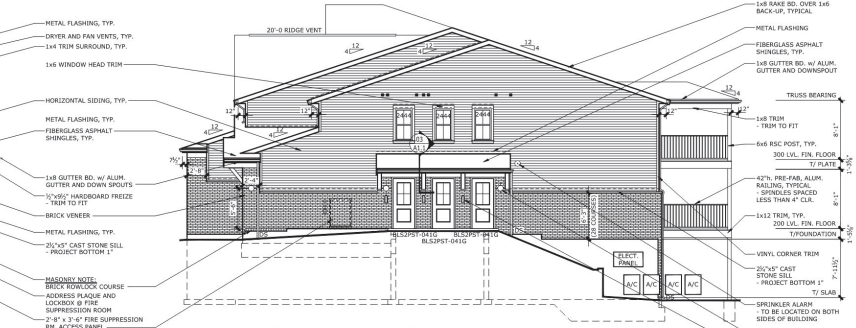
PLOT DATE: 11/29/2017 11:51 AM
 REASON FOR PLOT:



01 FRONT EXTERIOR
SCALE: 3/16" = 1'-0"



02 LEFT SIDE EXTERIOR
SCALE: 1/8" = 1'-0"



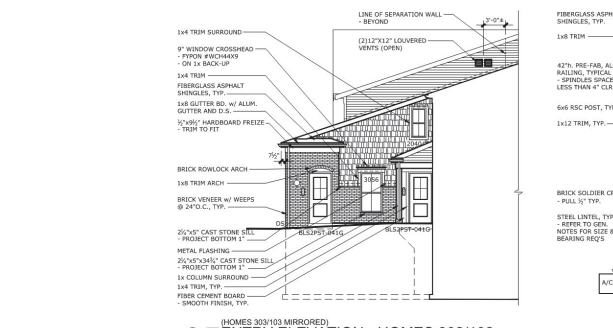
03 RIGHT SIDE EXTERIOR
SCALE: 1/8" = 1'-0"

DESIGN LOADS

FLOOR LOADS	LLS	SESD
300 FLOOR FRAMING	40 psf	20 psf
300 FLOOR FRAMING (TRUSSES)	40 psf	20 psf
200 FLOOR FRAMING (DOORS)	40 psf	20 psf
REAR DECKS	40 psf	20 psf
ROOF LOAD (TOTAL DESIGN LOAD)	37 psf	
SNOW (DESIGN LOAD)	20 psf	
WIND (DESIGN LOAD)	(3 Sec. Gusts)	0
EXPOSURE		B
EARTHQUAKE (DESIGN LOAD)	None	
See Section 01.1 "Seismic Load" - General seismic force-resisting system of cast base building that conforms to Section 2108 (Conventional Light Frame Construction) are not required to be analyzed as specified in Section 01.1.1.		

BUILDING ANALYSIS

- TWO STORY BUILDING (2 LEVELS)
- THIS BUILDING IS SPINDLING
- THROUGHOUT IN ACCORDANCE WITH IBC/AIA 1.9L
- USE GROUP: R-3 (One-Family)
- CONSTRUCTION TYPE: S-B



05 (HOMES 303/103 MIRROR) ENTRY ELEVATION - HOMES 302/102
SCALE: 1/8" = 1'-0"



04 REAR EXTERIOR
SCALE: 1/8" = 1'-0"

PREPARED BY: FISCHER ATTACHED HOMES III, LLC
DATE: 11/29/2017
REVISIONS: 01
FIELD SPEC. HOTLINE: (859) 344-2100
ADDRESS: 2000 W. BROADWAY, SUITE 200
PROJECT INFO: 2000 W. BROADWAY, SUITE 200
DATE: 11/29/2017
PROJECT COORDINATOR: JESSICA GIBSON, P.E.
PHONE NO.: (859) 344-2100
TEAM MANAGER: JESSICA GIBSON, P.E.
PHONE NO.: (859) 344-2100

BUILDING TYPE: 1-4 HOME GALLERY II
BUILDING #: ---
IN

FISCHER HOMES

Sheet No.
A1.0

Job No.
